# 2,5

2.5 An unknown immiscible liquid seeps into the bottom of an open oil tank. Some measurements indicate that the depth of the unknown liquid is 1.5 m and the depth of the oil (specific weight = 8.5 kN/m³) floating on top is 5.0 m. A pressure gage connected to the bottom of the tank reads 65 kPa. What is the specific gravity of the unknown liquid?

specific gravity of the unknown liquid?

$$P_{bottom} = (8_{oi})(5_{m}) + (8_{u})(1.5_{m}) \quad \text{where} \quad 8_{u} \sim \text{unknown liquid} \\
8_{u} = \frac{19_{oothorm} - (8_{oi})(5_{m})}{1.5_{m}} = 65 \times 10^{3} \frac{N}{m^{2}} - (8.5 \times 10^{3} \frac{N}{m^{2}})(5_{m}) \\
= 15 \times 10^{3} \frac{N}{m^{3}}$$

$$5G = \frac{8_{u}}{8_{u}} = \frac{15 \times 10^{3} \frac{N}{m^{3}}}{9.81 \times 10^{3} \frac{N}{m^{3}}} = \frac{1.53}{1.53}$$

2.9 Bathyscaphes are capable of submerging to great depths in the ocean. What is the pressure at a depth of 5 km, assuming that seawater has a constant specific weight of 10.1 kN/m³? Express your answer in pascals and psi.

$$p = \chi h + h_0$$
At the surface  $p_0 = 0$  so that
$$p = (10.1 \times 10^3 \frac{N}{m^3})(5 \times 10^3 m) = 50.5 \times 10^6 \frac{N}{m^2} = \frac{50.5 MPa}{2000}$$
Also,
$$p = (50.5 \times 10^6 \frac{N}{m^2})(1.450 \times 10^4 \frac{lb}{m^2}) = \frac{7320 psi}{2000}$$

2.27 Bourdon gages (see Video V2,4 and Fig. 2.13) are commonly used to measure pressure. When such a gage is attached to the closed water tank of Fig. P2.27 the gage reads 5 psi. What is the absolute air pressure in the tank? Assume standard atmospheric pressure of 14.7 psi.

$$p = 3h + p_{0}$$

$$p_{gage} - (\frac{12}{12}f_{\pm}) \delta_{k_{2}0} = p_{air}$$

$$p_{air} = (5\frac{16}{in.^{2}} + 14.7\frac{16}{in.^{2}}) - (1 + p_{\pm})(62.4\frac{16}{f_{\pm}})$$

$$\frac{144 + \frac{16}{f_{\pm}}}{f_{\pm}}$$

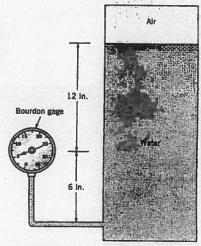
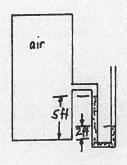


FIGURE P2.27

2).31 A water-filled U-tube manometer is used to measure the pressure inside a tank that contains air. The water level in the U-tube on the side that connects to the tank is 5 ft above the base of the tank. The water level in the other side of the U-tube (which is open to the atmosphere) is 2 ft above the base. Determine the pressure within the tank.

$$P_{air} + \delta_{H_{2}0} (5ff) - \delta_{H_{2}0} (2ff) = 0$$
or
$$P_{air} = -(3ff) \delta_{H_{2}0} = -(3ff) (62.4 \frac{16}{143})$$

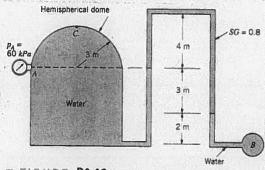
$$= -187 \frac{16}{142}$$



2..32 A barometric pressure of 29.4 in. Hg corresponds to what value of atmospheric pressure in psia, and in pascals?

(In psi) 
$$p = 8h = (847 \frac{16}{5t^3})(\frac{29.4}{12} ft ) \frac{15t^2}{144/in^2}) = 14.4 \text{ psia}$$
  
(In Pa)  $p = 8h = (133 \times 10^3 \frac{N}{m^3})(29.4 \text{ in})(2.540 \times 10^{-2} \frac{m}{\text{in}}) = 99.3 \text{ & Pa(abs)}$ 

2.39 A closed cylindrical tank filled with water has a hemispherical dome and is connected to an inverted piping system as shown in Fig. P2.39. The liquid in the top part of the piping system has a specific gravity of 0.8, and the remaining parts of the system are filled with water. If the pressure gage reading at A is 60 kPa, determine: (a) the pressure in pipe B, and (b) the pressure head, in millimeters of mercury, at the top of the dome (point C).



# FIGURE P2.89

(a) 
$$P_A + (SG)(\delta_{H_2O})(3m) + \delta_{H_2O}(2m) = P_B$$
  
 $P_B = GO k P_A + (0.8)(9.81 \times 10^3 \frac{N}{m^2})(3m) + (9.80 \times 10^3 \frac{N}{m^2})(2m)$   
 $= 103 k P_A$ 

(6) 
$$p_{c} = p_{A} - \delta_{H_{20}} (3m)$$

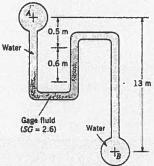
$$= b0 k P_{a} - (9.80 \times 10^{3} \frac{N}{m^{2}})(3m)$$

$$= 30.6 \times 10^{3} \frac{N}{m^{2}}$$

$$h = \frac{p_{c}}{\delta_{H_{3}}} = \frac{30.6 \times 10^{3} \frac{N}{m^{2}}}{133 \times 10^{3} \frac{N}{m^{3}}} = 0.230 m$$

$$= 0.230 m (\frac{10^{3} mm}{m}) = \frac{230 mm}{130 m}$$

2.90) Two pipes are connected by a manometer as shown in Fig. P2.40, Determine the pressure difference,  $p_A - p_B$ , between the pipes.



BFIGURE P2.40

$$P_{A} + \delta_{H_{2}O} (0.5m + 0.6m) - \delta_{gf} (0.6m) + \delta_{H_{2}O} (1.3m - 0.5m) = P_{B}$$
Thus,
$$P_{A} - P_{B} = \delta_{gf} (0.6m) - \delta_{H_{2}O} (0.5m + 0.6m + 1.3m - 0.5m)$$

$$= (2.6)(9.81 \frac{kN}{m^{3}})(0.6m) - (9.80 \frac{kN}{m^{3}})(1.9m)$$

$$= -3.32 \frac{kR}{m}$$

2.33 For the inclined-tube manometer of Fig. P2.43 the pressure in pipe A is 0.6 psi. The fluid in both pipes A and B is water, and the gage fluid in the manometer has a specific gravity of 2.6. What is the pressure in pipe B corresponding to the differential reading shown?

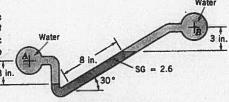


FIGURE P2.43

$$P_{A} + \delta_{\mu_{20}} \left(\frac{3}{12}ft\right) - \delta_{gf} \left(\frac{8}{12}ft\right) \sin 30^{\circ} - \delta_{\mu_{20}} \left(\frac{3}{12}ft\right) = P_{B}$$
(where  $\delta_{gf}$  is the specific weight of the gage fluid)

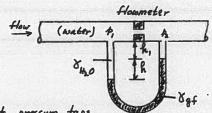
Thus,
$$P_{B} = P_{A} - \delta_{gf} \left(\frac{8}{12}ft\right) \sin 30^{\circ}$$

$$= \left(0.6 \frac{16}{10.5}\right) \left(144 \frac{\sin^{2}}{ft^{2}}\right) - \left(3.6\right) \left(62.4 \frac{16}{ft^{2}}\right) \left(\frac{8}{12}ft\right) \left(0.5\right) = 32.3 \frac{16}{ft^{2}}$$

$$= 32.3 \frac{16}{12} \left(144 \frac{\sin^{2}}{ft^{2}}\right) - (2.6) \left(62.4 \frac{16}{ft^{2}}\right) \left(\frac{8}{12}ft\right) \left(0.5\right) = 32.3 \frac{16}{ft^{2}}$$

#### 2.44

2.44 A flowrate measuring device is installed in a horizontal pipe through which water is flowing. A U-tube manometer is connected to the pipe through pressure taps located 3 in. on either side of the device. The gage fluid in the manometer has a specific weight of 112 lb/ft<sup>3</sup>. Determine the differential reading of the manometer corresponding to a pressure drop between the taps of 0.5 lb/in.<sup>2</sup>.



Let P, and P<sub>2</sub> be pressures at pressure taps.

Write manometer equation between P, and P<sub>2</sub>. Thus,

$$h = \frac{b_1 - b_2}{\delta_{gf} - \delta_{H_2O}} = \frac{\left(0.5 \frac{lb}{in.^2}\right) \left(144 \frac{in.^2}{ft^2}\right)}{112 \frac{lb}{ft^3} - 62.4 \frac{lb}{ft^3}}$$

2.53 The inverted U-tube manometer of Fig. P2.53 contains oil (SG = 0.9) and water as shown. The pressure differential between pipes A and B,  $p_A - p_B$ , is -5 kPa. Determine the differential reading, h.

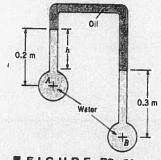
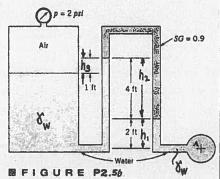


FIGURE P2.59

$$P_{A} - \delta_{H_{20}}(0,2m) + \delta_{i,l}(k) + \delta_{H_{20}}(0,3m) = P_{B}$$
Thus,
$$h = \frac{(P_{B} - P_{A}) + \delta_{H_{20}}(0,2m) - \delta_{H_{20}}(0,3m)}{\delta_{i,l}}$$

$$= \frac{5 \times 10^{3} \frac{N}{m^{2}} - (9.80 \times 10^{3} \frac{N}{m^{3}})(0,1m)}{8.95 \times 10^{3} \frac{N}{m^{3}}} = 0.449 \text{ m}$$

2.56 Determine the pressure of the water in pipe A shown in Fig. P2.56 if the gage pressure of the air in the tank is 2 psi.



$$\rho_{A} - \delta_{W}^{\prime} h_{1} - (0.9 \delta_{W}) h_{2} + \delta_{W}^{\prime} h_{3} = \rho_{air}$$
or
$$\rho_{A} = \rho_{air} + \delta_{W}^{\prime} (h_{1} + 0.9 h_{2} - h_{3})$$

$$= 2 \frac{lb}{in^{2}} \left( \frac{144 in^{2}}{fl^{2}} \right) + 62.4 \frac{lb}{fl^{2}} \left( fl + 0.9 (4fl) - 1fl \right)$$

$$= 575 \frac{lb}{fl^{2}}$$

2.58 In Fig. P2.50 pipe A contains gasoline (SG = 0.7), pipe B contains oil (SG = 0.9), and the manometer fluid is mercury. Determine the new differential reading if the pressure in pipe A is decreased 25 kPa, and the pressure in pipe B remains constant. The initial differential reading is 0.30 m as shown.

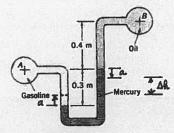


FIGURE P2.59

For the initial configuration:  

$$p_A + \delta_{gas}(0.3m) - \delta_{Hg}(0.3m) - \delta_{oil}(0.4m) = P_D$$
 (1)

With a decrease in p to p' gage fluid levels change us shown on figure. Thus, for final configuration:

where all lengths are in m. Subtract Eq.(2) from Eq.(1) to obtain,

$$p_{A} - p_{A}' + \delta_{gas}(a) - \delta_{Hg}(0.3 - hh) + \delta_{oi}(a) = 0$$
 (3)

Since 
$$2a + \Delta h = 0.3$$
 (see figure) then
$$a = \frac{0.3 - \Delta h}{2}$$

and from Eq.(3)

Thus,

$$Sh = \frac{P_{A} - P_{A}' + \delta_{gas}(0.15) - \delta_{Hg}(0.3) + \delta_{0il}(0.15)}{-\delta_{Hg} + \frac{\delta_{gas}}{2} + \frac{\delta_{0il}}{2}}$$

and with PA - PA' = 25 k Pa

$$\Delta h = 25 \frac{RN}{m^2} + (0.7)(9.81 \frac{kN}{m^2})(0.16m) - (133 \frac{kN}{m^2})(0.3m) + (0.9)(9.81 \frac{kN}{m^2})(0.16m) - (133 \frac{kN}{m^2})(0.3m) + (0.9)(9.81 \frac{kN}{m^2})(0.16m) - (133 \frac{kN}{m^2}) + (0.9)(9.81 \frac{kN}{m^2})$$

= 0.100 m

2.59 The mercury manometer of Fig. P2.59 indicates a differential reading of 0.30 m when the pressure in pipe A is 30 mm Hg vacuum. Determine the pressure in pipe B.

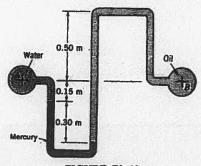


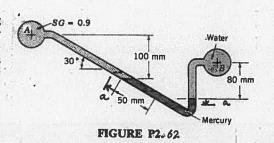
FIGURE P2.59

$$p_B + \delta_{oil} (0.15 m + 0.30 m) - \delta_{Hg} (0.3 m) - \delta_{H_{20}} (0.15 m) = p_A$$
  
Where  $p_A = -\delta_{Hg} (0.030 m)$ 

Thus,
$$\frac{1}{70} = -\delta_{Hg} \left(0.030 \text{ m}\right) - \delta_{0i} \left(0.45 \text{ m}\right) + \delta_{Hg} \left(0.3 \text{ m}\right) + \delta_{H_{20}} \left(0.15 \text{ m}\right)$$

$$= -\left(133 \frac{k_N}{m_B}\right) \left(0.030 \text{ m}\right) - \left(8.95 \frac{k_N}{m_B}\right) \left(0.45 \text{ m}\right) + \left(133 \frac{k_N}{m_B}\right) \left(6.3 \text{ m}\right) + \left(9.80 \frac{k_N}{m_B}\right) \left(0.15 \text{ m}\right)$$

2.62 Determine the new differential reading along the inclined leg of the mercury manometer of Fig. P2.62, if the pressure in pipe A is decreased 10 kPa and the pressure in pipe B remains unchanged. The fluid in A has a specific gravity of 0.9 and the fluid in B is water.



For the initial configuration :

where all lengths are in m. When p decreases left column moves up a distance, a, and vignt column moves down a distance, a, as shown in figure. For the final configuration:

$$Y_{\mu_{20}}(0.08+a) = -\beta_{B}$$
 (2)

where p is the new pressure in pipe A. Subtract Eq.(2) from Eq.(1) to obtain

Thus,

$$\alpha = \frac{-(p_{A} - p_{A}^{\prime})}{\gamma_{A} \sin 30^{\circ} - \gamma_{H_{2}} (\sin 30^{\circ} + 1) + \gamma_{H_{2}0}}$$

For \$ - \$ = 10 & Pa

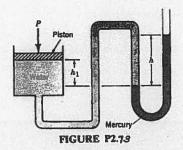
$$a = \frac{-10 \frac{4 N}{m^2}}{(0.9)(9.81 \frac{4 N}{m^2})(0.5) - (133 \frac{4 N}{m^3})(0.5+1) + 9.80 \frac{4 N}{m^3}}$$

= 0.0540 m

New differential reading,  $\Delta h$ , measured along inclined tube is equal to  $\Delta h = \frac{a}{\sin 30^{\circ}} + 0.05 + a$ 

 $= \frac{0.0540 \text{ m}}{6.5} + 0.05 \text{ m} + 0.0540 \text{ m} = \frac{0.212 \text{ m}}{6.5}$ 

2.73 A piston having a cross-sectional area of  $0.07 \text{ m}^2$  is located in a cylinder containing water as shown in Fig. P2.73. An open U-tube manometer is connected to the cylinder as shown. For  $h_1 = 60 \text{ mm}$  and h = 100 mm, what is the value of the applied force, P, acting on the piston? The weight of the piston is negligible.



For equilibrium, P= p Ap where p is the pressure acting on piston and Ap is the area of the piston. Also,

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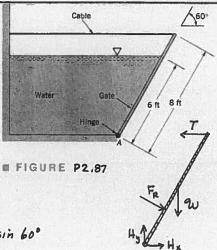
$$P_{p} = \delta_{H_{q}} h - \delta_{H_{20}} h,$$

$$= (133 \frac{kN}{m^{3}})(0.100 m) - (9.80 \frac{kN}{m^{3}})(0.060 m)$$

$$= 12.7 \frac{kN}{m^{2}}$$

$$P = (12.7 \times 10^{3} \frac{N}{m^{2}})(0.07 \text{ m}^{2}) = \frac{889 \text{ N}}{100}$$

2.87 A homogeneous, 4-ft-wide, 8-ft-long rectangular gate weighing 800 lb is held in place by a horizontal flexible cable as shown in Fig. P2.97, Water acts against the gate which is hinged at point A. Friction in the hinge is negligible. Determine the tension in the cable.



Thus,
$$F_{R} = (62.4 \frac{16}{443})(\frac{64}{2})(\sin 60^{2})(64t \times 44t)$$

$$= 3890 \frac{1}{6}$$

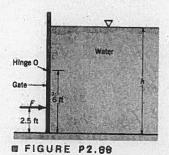
To locate 
$$F_R$$
,  
 $y_R = \frac{I_{xc}}{y_c A} + y_c$  where  $y_c = 3ft$ 

So that 
$$\frac{1}{72} \frac{(4ft)(1ft)^3}{(3ft)(1ft)(4ft)} + 3ft = 4.6 ft$$

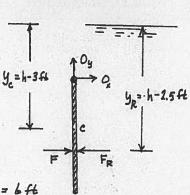
$$ZM_{H} = 0$$
and
$$T (8ft)(sin 60°) = 9v (4ft)(cos60°) + F_{R} (2ft)$$

$$T = \frac{(800 lb)(4ft)(cos60°) + (8890 lb)(2ft)}{(8ft)(sin 60°)}$$

2.38 A rectangular gate 6 ft tall and 5 ft wide in the side of an open tank is held in place by the force F as indicated in Fig. P2.98. The weight of the gate is negligible, and the hinge at 0 is frictionless. (a) Determine the water depth, h, if the resultant hydrostatic force of the water acts 2.5 ft above the bottom of the gate, i.e., it is collinear with the applied force F. (b) For the depth of part (a), determine the magnitude of the resultant hydrostatic force. (c) Determine the force that the hinge puts on the gate under the above conditions.



(a)  $y_R - y_c = \frac{I_{xc}}{y_c A}$   $(h-2.5ft) - (h-3.0ft) = \frac{1}{12} (5ft) (6ft)^3$  (n-3ft) (6ft\*5ft)0.5ft =  $\frac{3ft}{h-3ft}$ 



(b) 
$$F_R = 8 h_c A$$
 Where  $h_c = h - 3ft = 6ft$   
=  $(62.4 \frac{16}{ft^3}) (6ft) (6ft \times 5ft)$   
=  $11,200 lb$ 

2.95 A gate having the cross section shown in Fig. P2.95 closes an opening 5 ft wide and 4 ft high in a water reservoir. The gate weighs 500 lb and its center of gravity is 1 ft to the left of AC and 2 ft above BC. Determine the horizontal reaction that is developed on the gate at C.

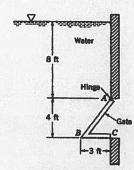


FIGURE P2.95

$$F_{1} = 8 h_{c1} A_{1} \quad \text{where } h_{c1} = 8 \text{ ft} + 2 \text{ ft}$$

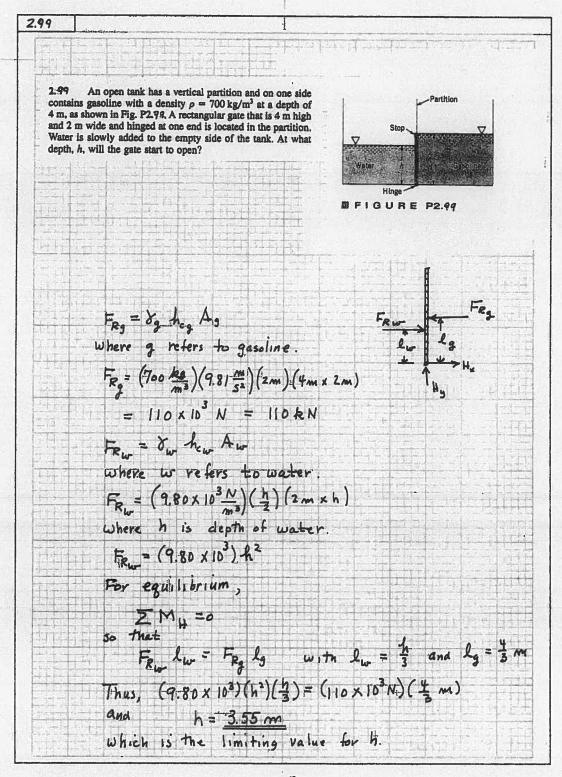
Thus,
$$F_{1} = (62.4 \frac{16}{fc^{2}}) (10 \text{ ft}) (5 \text{ ft} \times 5 \text{ ft})$$

$$= 15,600 \text{ lb}$$

To locate  $F_{1}$ ,
$$y_{1} = \frac{I_{AC}}{y_{c1}A_{1}} + y_{c1}$$
where  $y_{c1} = \frac{8 \text{ ft}}{4} + 2.5 \text{ ft} = 12.5 \text{ ft}$ 

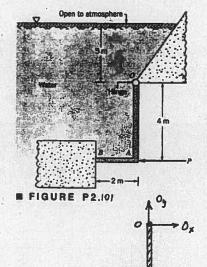
$$F_{2} = \frac{1}{2} (5 \text{ ft}) (5 \text{ ft})^{3} + 12.5 \text{ ft} = 12.67 \text{ ft}$$

Also,
$$F_{2} = \frac{1}{2} A_{2} \quad \text{where } \frac{1}{2} = \frac{1}{2} A_{2} \quad \text{where$$



2.10) The rigid gate, OAB, of Fig. P2,10) is hinged at O and rests against a rigid support at B. What minimum horizontal force, P, is required to hold the gate closed if its width is 3 m? Neglect the weight of the gate and friction in the hinge. The back of the gate is exposed to the atmosphere.

Thus, 
$$F_1 = 8 h_{c_1} A_1$$
, where  $h_{c_1} = 5 m$   
Thus,  $F_1 = (9800 \frac{N}{m^3})(5m)(4m \times 3m)$   
 $= 5.88 \times 10^5 N$   
 $F_2 = 8 h_{c_2} A_2$  where  $h_{c_2} = 7 m$   
so that  
 $F_2 = (9800 \frac{N}{m^3})(7m)(2m \times 3m)$   
 $= 4.12 \times 10^5 N$ 



P

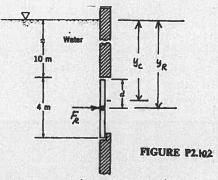
To locate 
$$F_{i,j}$$
  
 $Y_{R_i} = \frac{I_{xc}}{Y_{c,k_i}} + Y_{c,j} = \frac{\frac{1}{2}(3m)(4m)^3}{(5m)(4m)^3} + 5m = 5.267m$ 

The force  $F_2$  acts at the center of the AB section. Thus,  $Z M_0 = 0$ 

$$P = \frac{(5.88 \times 10^{5} N)(2.267m) + (4.12 \times 10^{5} N)(1m)}{4m}$$

$$= 436 kN$$

2.j02. A rectangular gate that is 2 m wide is located in the vertical wall of a tank containing water as shown in Fig. P2.l02.It is desired to have the gate open automatically when the depth of water above the top of the gate reaches 10 m. (a) At what distance, d, should the frictionless horizontal shaft be located? (b) What is the magnitude of the force on the gate when it opens?



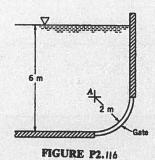
(a) As depth increases the center of pressure moves toward the centraid of the gate. If we locate hinge at y, when depth = 10m +d, the gate will open automatically for any further increase in depth.

$$y_R = \frac{I_{XC}}{y_c A} + y_c = \frac{\frac{1}{12} (2m)(4m)^3}{(12m)(2m \times 4m)} + 12m = 12.11m$$

then

(b) For the depth shown,

2.116 A4-m-long curved gate is located in the side of a reservoir containing water as shown in Fig. P2.116. Determine the magnitude of the horizontal and vertical components of the force of the water on the gate. Will this force pass through point A? Explain.



For equilibrium,

$$\Sigma F_{X} = 0$$

or

 $F_{W} = F_{Z} = 8 \, h_{C2} \, A_{2} = 8 \, (4m + 1m) (2m \times 4m)$ 

For that

 $F_{H} = (9.80 \, \frac{kN}{m^{3}}) (5m) (8m^{2}) = \frac{392 \, kN}{392 \, kN}$ 

Similarly,

 $\Sigma F_{Y} = 0$ 
 $F_{V} = F_{I} + 9V$  where:

 $F_{I} = [8 \, (4m)] (2m \times 4m) = (9.80 \, \frac{kN}{m^{2}}) (4m) (8m^{2})$ 

W= 8+= (9.80 AN) (47 m3)

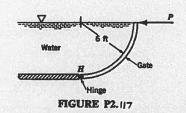
Find 
$$F_1$$
 $F_2$ 
 $F_3$ 
 $F_4$ 
 $F_5$ 
 $F_6$ 
 $F_6$ 
 $F_7$ 
 $F_8$ 
 $F_$ 

Thus, 
$$F_V = (9.80 \frac{kN}{m^3}) \left[ 32 m^3 + 4\pi m^3 \right] = 437 kN$$

(Note: Force of water on gate will be opposite in direction to ) that shown on figure.

The direction of all differential forces acting on the curved surface is perpendicular to surface, and therefore, the resultant must pass through the intersection of all these forces which is at point A. Yes.

2.117 The 18-ft-long gate of Fig. P2.117 is a quarter circle and is hinged at H. Determine the horizontal force, P, required to hold the gate in place. Neglect friction at the hinge and the weight of the gate.



For equilibrium (from free-body-diagram of fluid mass),

$$ZF_{L}=0$$

So that

$$F_{H}=F_{I}=8h_{e}, A,$$

$$=(62.4\frac{16}{4t^{3}})(\frac{6ft}{2})(6ft\times 18ft)=20,20016$$

Similarly,

 $ZF_{V}=0$ 

So That

$$F_{V}=W=8_{Hz0}\times(volume\ of\ fluid)=(62.4\frac{16}{ft^{3}})\left[\frac{\pi}{4}(6ft)^{2}18ft\right]=31,80016$$

Also,

 $X_{I}=\frac{4(6ft)}{3\pi}=\frac{8}{11}ft$ 

(see Fig. 2.18e)

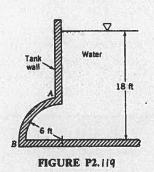
and

 $y_{I}=\frac{6ft}{3}=2ft$ 

For equilibrium (from free-body-diagram of gate)
$$\sum M_{o} = o$$
so that
$$P(6ft) = F_{H}(y_{i}) + F_{V}(x_{i})$$
or
$$P = \frac{(20,200 \text{ lb})(2ft) + (31,800 \text{ lb})(\frac{3}{11}ft)}{6.54} = 20,200 \text{ lb}}$$

and

2.//9 A tank wall has the shape shown in Fig. P2.//9 Determine the horizontal and vertical components of the force of the water on a4-ft length of the curved section AB.



$$F_{1} = 8 h_{e1} A_{1}$$

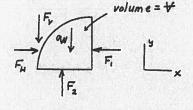
$$= (62.4 \frac{b}{ft^{3}})(15ft)(6ftx4ft)$$

$$= 22,500 lb$$

$$F_{2} = 8 h_{e2} A_{2}$$

$$= (62.4 \frac{lb}{ft^{3}})(18ft)(6ftx4ft)$$

$$= 27,000 lb$$



$$W = 8 \forall = (62.4 \frac{b}{ft})(\frac{1}{4})(\pi)(6ft)^{2}(4ft)$$
= 7060 | b

For equilibrium,

$$ZF_{x}=0$$

so that

 $F_{H}=F_{1}=\frac{22,500}{6}$  on tank

2.145 When a hydrometer (see Fig. P2J45 and Video V2.8) having a stem diameter of 0.30 in. is placed in water, the stem protrudes 3.15 in. above the water surface. If the water is replaced with a liquid having a specific gravity of 1.10, how much of the stem would protrude above the liquid surface? The hydrometer weighs 0.042 lb.

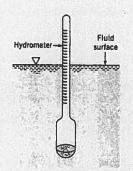


FIGURE P2.145

When the hydrometer is floating its weight, 20, is balanced by the buoyant force, Fig. For equilibrium,

$$\sum_{vertical} F_{vertical} F_{vertical}$$
Thus, for water
$$F_{vertical} F_{vertical} F_{vertical}$$
Where  $F_{vertical} F_{vertical} F_{vertical}$ 
where  $F_{vertical} F_{vertical} F_{vertical}$ 
(1)

where  $F_{vertical} F_{vertical} F_{vertical}$ 
(2)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(3)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(4)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(5)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(6)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(6)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(7)

Combining  $F_{vertical} F_{vertical} F_{vertical}$ 
(8)

(con't)

# 2.145 (Con't)

From Eq. (1)

$$\psi_{1} = \frac{2\nu}{8\mu_{20}} = \frac{0.042 \, lb}{62.4 \, lb} = 6.73 \times 10^{-4} \, \text{ft}$$

So that from Eq. (3)

 $\psi_{2} = \frac{6.73 \times 10^{-4}}{1.10} \, \text{ft}^{3} = 6.12 \times 10^{-4} \, \text{ft}^{3}$ 

Thus,  $\psi_{1} - \psi_{2} = (6.73 - 6.12) \times 10^{-4} \, \text{ft}^{3} = 0.6 \, \text{fx} \times 10^{-4} \, \text{ft}^{3}$ 
To obtain this difference the change in length  $\Delta l$ , is

 $\left(\frac{T}{4}\right)(0.30 \, \text{in.})^{3} \Delta l = (0.61 \times 10^{-4} \, \text{ft}^{3})(1728 \, \frac{\text{in.}^{3}}{\text{ft}^{3}})$ 
With the new liquid the stem would protrude

 $3.15 \, \text{in.} + 1.49 \, \text{in.} = 4.64 \, \text{in.} \, \text{above the surface}$ 

2.147 (See Fluids in the News article titled "Concrete canoe," Section 2.11.1.) How much extra water does a 147-lb concrete canoe displace compared to an ultralightweight 38-lb Kevlar canoe of the same size carrying the same load?

For equilibrium,

and 20 = FB = 8 + and + is displaced volume.

For concrete canoe,

Vc = 2.36 ft3

For Kevlar canoe,

3816 = (62.4 16.) +

Vi = 0.609 ft3

Extra water displacement =  $2.36 \text{ ft}^3 - 0.609 \text{ ft}^3$ =  $1.75 \text{ ft}^3$